

GENERAL NOTES

- CHECK ALL DIMENSIONS ON STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL DRAWINGS. REPORT ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK. DO NOT SCALE THESE DRAWINGS.
- FOR THE EXISTING STRUCTURE, REFER TO THE FOLLOWING STRUCTURAL DRAWINGS:
 - S1-91, S2-91, S2-92, S3-91 TO S3-20, S4-91 TO S4-12, DATED MAY 14, 1993, PREPARED BY ROBERT HALSALL AND ASSOCIATES LTD., PROJECT #9034
 - S1 TO S18, DATED 99-11-12, PREPARED BY HALSALL ASSOCIATES LTD., PROJECT #9060
- STRUCTURAL PLANS SHOW BEARING WALLS AND COLUMNS BELOW THE FLOOR OR ROOF STRUCTURE WITH CONTINUOUS LINES. WALLS AND COLUMNS ABOVE THE FLOOR ARE SHOWN WITH DASHED LINES.
- "T" SECTIONS ON THE DRAWINGS REFER TO TYPICAL DETAILS ON DRAWINGS S1001 AND S1002. THEY SHOW STRUCTURAL INTENT RATHER THAN ACTUAL CONDITIONS FOR THIS PROJECT.
- CARRY ALL FOOTINGS DOWN TO STRATA CAPABILITY OF SUPPORTING THE DESIGN BEARING PRESSURES NOTED, BUT NOT LESS THAN 600 mm BELOW ORIGINAL GRADE, AND FOR EXTERIOR FOOTINGS NOT LESS THAN REQUIRED TO PROVIDE A MINIMUM OF 1200 mm FROST PROTECTION.
- PROTECT FOOTINGS, WALLS, SLABS-ON-GRADE AND ADJACENT SOIL AGAINST FREEZING AND FROST ACTION AT ALL TIMES DURING CONSTRUCTION.
- THE LINE OF SLOPE BETWEEN ADJACENT EXCAVATIONS FOR FOOTINGS OR TRENCHES SHALL NOT EXCEED A RISE OF 7 IN A RUN OF 10.
- FOOTING STEPS SHALL BE A MINIMUM OF 1200 mm APART. MAXIMUM STEP APPROXIMATELY 600 mm.
- CENTRE FOOTINGS AND PIERS UNDER CENTRE OF COLUMNS, UNLESS OTHERWISE NOTED. PROVIDE MINIMUM 100 mm CLEARANCE BETWEEN WALLS AND COLUMNS.
- DO NOT BACKFILL AGAINST WALLS RETAINING EARTH UNTIL ELEMENTS PROVIDING LATERAL SUPPORT ARE COMPLETED. PLACE BACKFILL SIMULTANEOUSLY ON BOTH SIDES OF OTHER WALLS BELOW GRADE.
- HORIZONTAL CONSTRUCTION JOINTS IN CONCRETE WALLS ARE NOT PERMITTED, EXCEPT WHERE SHOWN ON THESE DRAWINGS. LEAVE CHANGES AND POCKETS IN WALLS FOR SEATING OF SLABS AND BEAMS.
- REINFORCE ALL SIDES OF OPENINGS IN CONCRETE WALLS. LENGTH OF BARS EQUAL TO OPENING DIMENSION PLUS 600 mm EACH SIDE. USE 2 - 10 FOR WALL THICKNESS UP TO 200 mm, AND 2 - 20 FOR WALLS OVER 200 mm.
- REINFORCING FOR CONCRETE WALLS NOT COVERED BY SECTION OR PLAN SHALL BE AS FOLLOWS:

150 mm MAXIMUM WALL:	10 @ 300 H + 10 @ 400 V IN CENTRE
200 mm MAXIMUM WALL:	10 @ 500 H + 10 @ 500 V
250 mm MAXIMUM WALL:	10 @ 400 H + 10 @ 500 V
300 mm MAXIMUM WALL:	10 @ 300 H + 10 @ 400 V
THICKER WALL:	15 @ 300 H + 15 @ 400 V
- REINFORCING FOR CONCRETE CURBS NOT COVERED BY SECTION OR PLAN SHALL BE 10#400 DOWELS + 2-10#.
- REINFORCING FOR CONCRETE BASES UNDER EQUIPMENT NOT COVERED BY SECTION OR PLAN SHALL BE 10#400 EA. WAY PLACED 50mm BELOW TOP OF CONCRETE.
- UNLESS OTHERWISE SPECIFIED ON PLANS, PROVIDE TEMPERATURE REINFORCING FOR FRAMED ONE-WAY SLABS IN ACCORDANCE WITH TYPICAL DETAILS.
- BARs MARKED CONTINUOUS SHALL BE TERMINATED IN HOOKS AND DEVELOPED BY CLASS B LAPS WHERE SPECIFIED.
- PROVIDE CONTINUOUS GALVANIZED VERTICAL DOVETAIL ANCHOR SLOTS IN ALL CONCRETE SURFACES AT ADJUTING MASONRY WALLS AND AT 600 mm CENTRES IN ALL CONCRETE SURFACES WITH MASONRY VENEER.
- STANDARD LINTELS:

PROVIDE STANDARD LINTELS OVER ALL OPENINGS IN MASONRY WALLS AND PARTITIONS AS FOLLOWS:

FOR WALLS REQUIRING MASONRY BLOCK LINTELS, FILL 200 mm DEEP LINTEL BLOCKS WITH 20 MPa CONCRETE REINFORCED WITH 1-10 TOP AND BOTTOM FOR EACH 100 mm OF WALL THICKNESS, OR PORTION THEREOF, FOR OPENINGS UP TO 1200 mm wide. BETWEEN 1200 mm and 2000 mm, USE 1 - 15 AS ABOVE.

FOR WALLS REQUIRING STRUCTURAL STEEL LINTELS, SEE L1/S1002 AND L2/S1002.

CHECK ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR OPENINGS REQUIRING STANDARD LINTELS WHICH ARE NOT NECESSARILY SHOWN ON THE STRUCTURAL DRAWINGS.

SPECIAL LINTELS: PROVIDE SPECIAL LINTELS AS PER LINTEL SCHEDULE AT LOCATIONS SHOWN ON PLAN.
- UNLESS OTHERWISE NOTED, FILL ALL CHANNEL BLOCK BOND BEAMS WITH 20 MPa CONCRETE REINFORCED WITH 1 - 10 TOP AND BOTTOM CONTINUOUS.
- UNLESS OTHERWISE NOTED, ALL BEARING BEAMS SHALL HAVE A MINIMUM BEARING OF 200 mm, AND ALL CONCRETE SLABS SHALL HAVE A MINIMUM BEARING OF 100 mm. JOISTS IN MASONRY UNITS UNDER BEAMS AND JOISTS SHALL BE PREFILLED WITH GROUT FOR A MINIMUM VERTICAL DEPTH OF 600 mm AND A LENGTH OF 400 mm. UNLESS OTHERWISE NOTED, USE 75# SOLID BLOCKS FOR FILLING. DO NOT USE MORTAR TO FILL MASONRY UNITS.
- MINIMUM CONCRETE COVER TO REINFORCING BARS, IN mm, UNLESS OTHERWISE NOTED:

FOOTINGS	75 TO BOTTOM BARS, 50 TO TOP BARS
PIERS	50 TO TIES
CASSIONS	75 TIES
CAPS	75 TO BOTTOM BARS, 50 TO TOP BARS
CRACK BEAMS	50 TO STIRRUPS
COLUMNS	40 TO TIES
WALLS	40 TO SURFACES EXPOSED TO GROUND OR OUTSIDE.
SLABS	20 TO PROTECTED SURFACES
PARKING & RAMP SLABS	40 TO TOP BARS AND 30 TO BOTTOM BARS
BEAMS	40 TO STIRRUPS

MATERIAL AND DESIGN DATA (FOR ALL BUILDINGS UNLESS NOTED)

- STRUCTURAL DESIGN IS IN ACCORDANCE WITH ONTARIO BUILDING CODE 1997, INCLUDING AMENDMENTS.
- FOOTING DESIGN BEARING PRESSURE IS 500 kPa (BLOCK A), 400 kPa (BLOCKS F-1/F-2) AND 350 kPa (PARKING P2). CASSION DESIGN BEARING PRESSURE IS 2000 kPa.
- SEE SOILS REPORT PREPARED BY: PETER MACCALLUM LTD. REPORT NUMBER: 99T101A, DATED: AUGUST 24, 2001, SUPPLEMENTED BY FAXES DATED AUG. 31, 2001 AND SEP. 12, 2001. ADDITIONAL DESIGN GUIDELINES DATED NOVEMBER 02, 2001, AND LETTER DATED MAY 14, 2002.
- CONCRETE SPECIFIED COMPRESSIVE STRENGTH, f_c , IS 25 MPa EXCEPT FOR:
 - ALL BLOCK A FRAMED SLABS AND BEAMS $f_c = 30$ MPa
 - ALL PARKING P2 CONCRETE COLUMNS, SLABS, WALLS, FOOTINGS: $f_c = 35$ MPa, CLASS C1 EXPOSURE
 - COLUMNS - AS SHOWN ON COLUMN SCHEDULES
 - SHEAR WALLS - AS SHOWN ON SHEAR WALL SCHEDULE
 - OTHER LOCATIONS NOTED ON PLANS, SCHEDULES AND SPECIFICATIONS
- REINFORCING STEEL: CAN/CSA C30.18M - GRADE 400R
- STRUCTURAL STEEL: CAN/CSA C40.21M
 - WIDE FLANGES: 350 W
 - HSS SECTIONS: 300W
 - CLASS H FOR 102 mm OR LARGER SECTIONS
 - CLASS C FOR SMALLER SECTIONS
 - ANCHOR BOLTS: 300W
 - ALL OTHER STEEL: 300W
- STRUCTURAL STEEL: ASTM A500, GRADE C (ONLY FOR HSS SECTIONS WITH SIZE LARGER THAN 305 mm)
- STRUCTURAL MASONRY:
 - HOLLOW BLOCK: CSA A165.1 - 1/15/1/4
 - SOLID BLOCK: CSA A165.1 - 5/15/1/4
 - MORTAR: CSA A173M - TYPE S
 - GROUT FOR BLOCK CORERS: CSA A173M - COARSE GROUT
 - 1:3.2 CEMENT/SAND/PEA-STONE BY VOLUME WITH 200 mm SLUMP
 - ASSUMED MASONRY COMPRESSIVE STRENGTH, f_m , IS:

HOLLOW BLOCK	= 9.8 MPa
ASSUMED HOLLOW BLOCK	= 7.5 MPa
SOLID BLOCK	= 7.5 MPa
 - ASSUMED FLEXURAL TENSILE BOND STRENGTH AT MORTAR JOINTS IS:

CONCRETE BRICK & BLOCK	= 0.45 MPa
GROUTED HOLLOW BLOCK	= 0.70 MPa
- GLUE LAMINATED STRUCTURAL UNITS: DOUGLAS FIR LARCH
 - BEAMS, PURLINS AND GIRTS: BENDING GRADE 20F-E
 - CORNER COLUMNS AND OUTRIGGER TRUSSES: COMPRESSION GRADE 16C-E
 - CLAMBER CONNECTIONS FOR NOMINAL LOADS ONLY: BENDING GRADE 20F-E
 - SERVICE GRADE: EXTERIOR
 - APPROACHES: EXTERIOR
- DESIGN LOADS FOR BUILDING STRUCTURE:
 - GRAVITY LOADS AS SHOWN ON PLANS
 - GROUND SNOW LOAD AND ASSOCIATED RAIN LOAD:

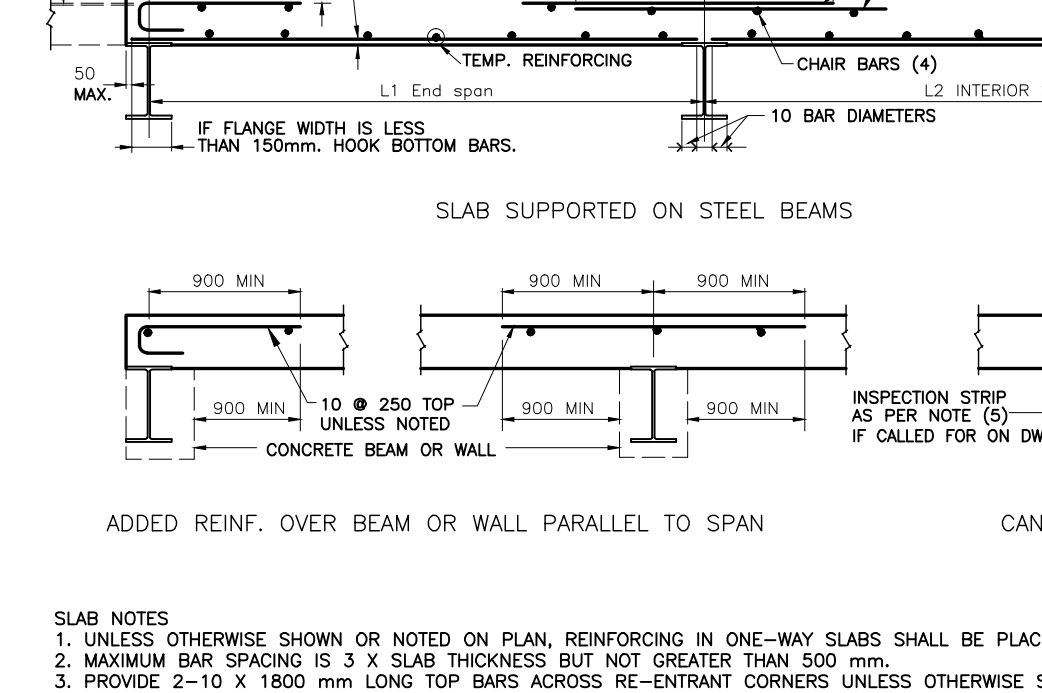
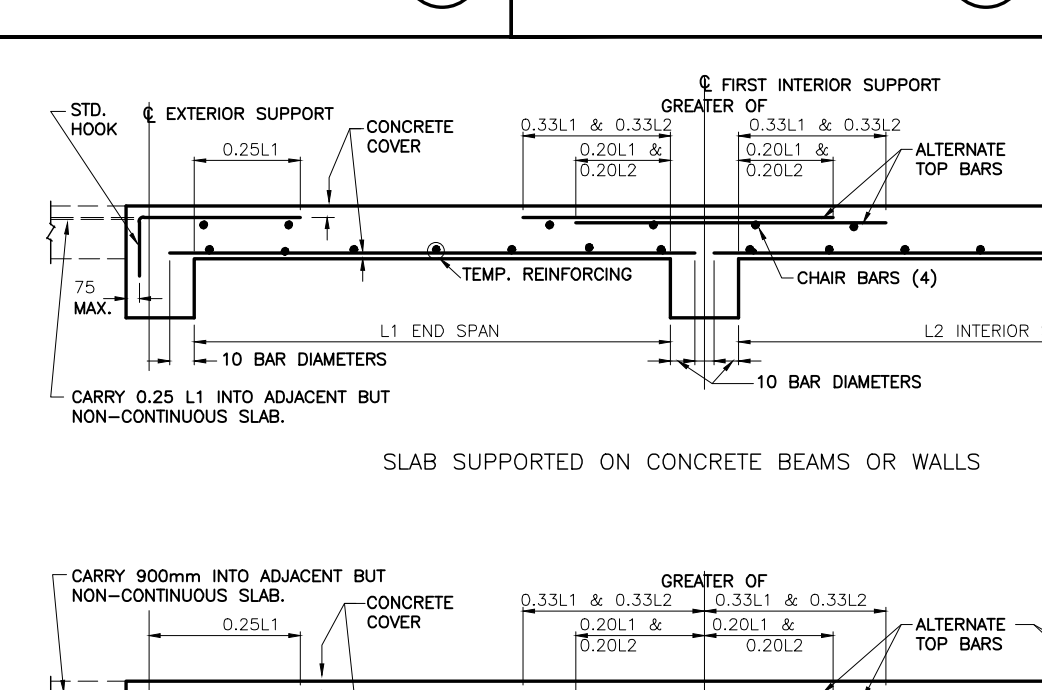
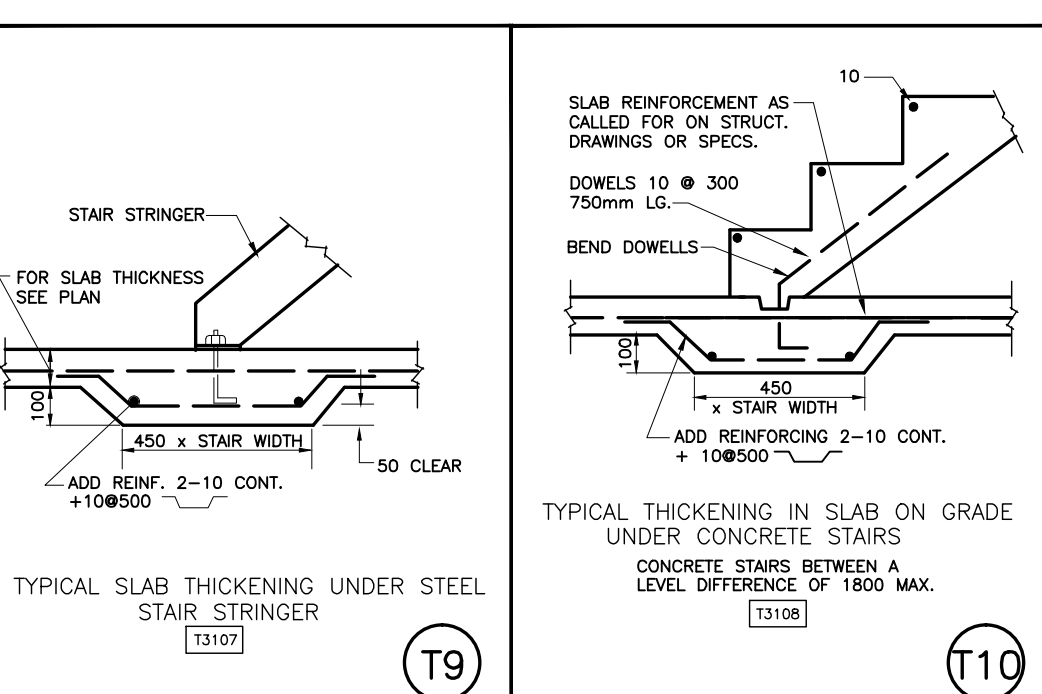
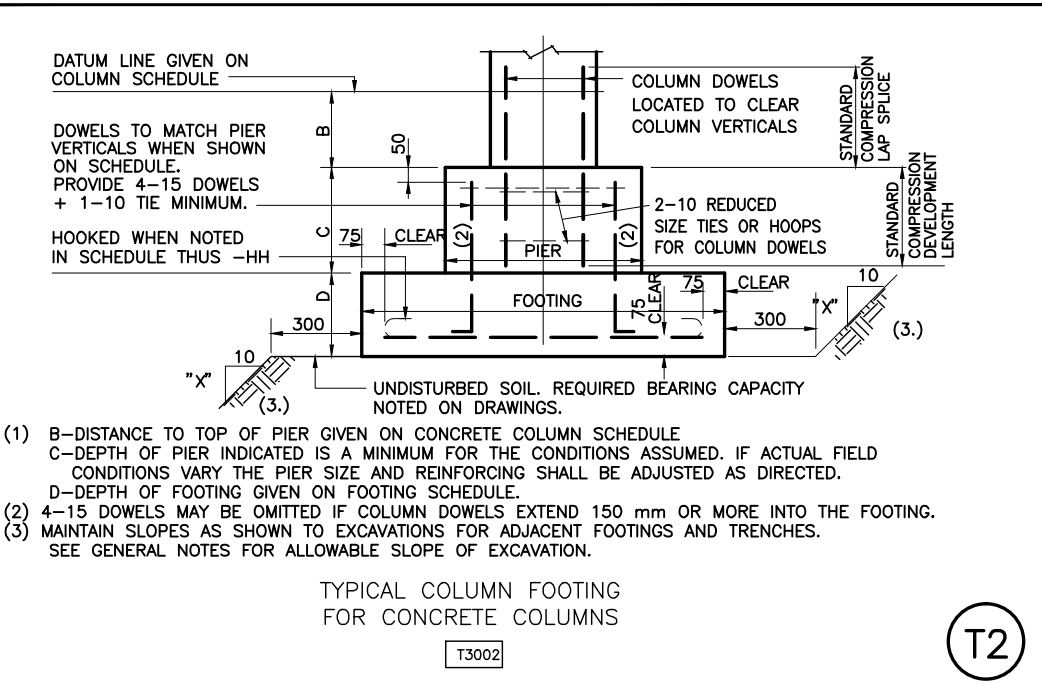
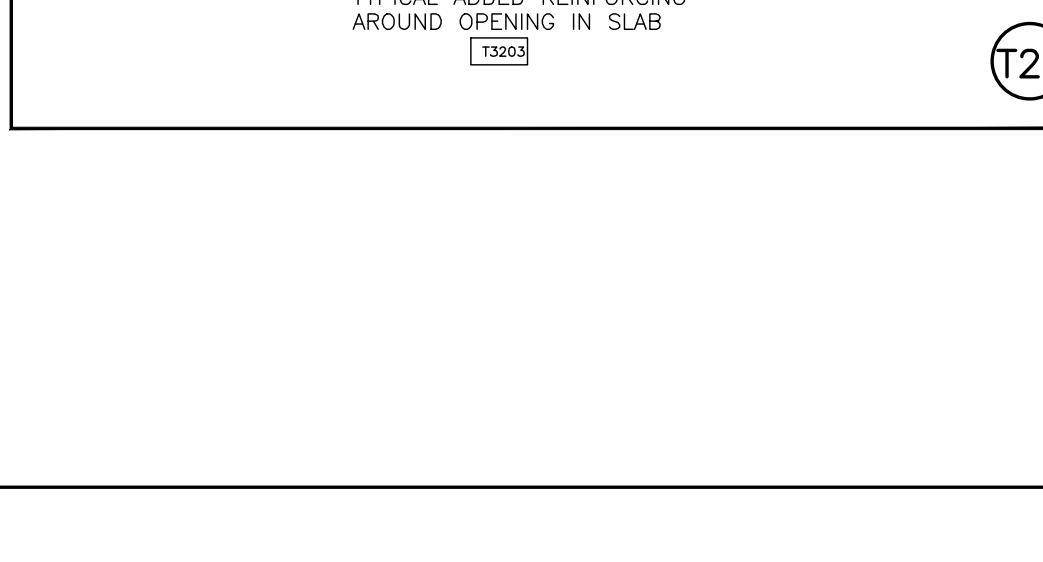
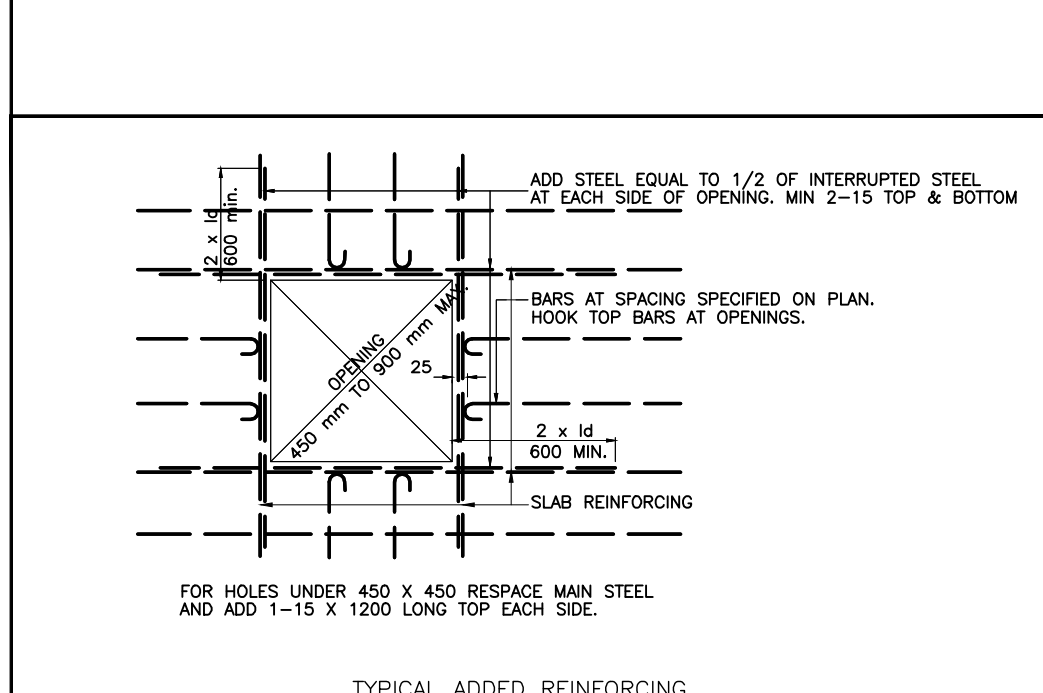
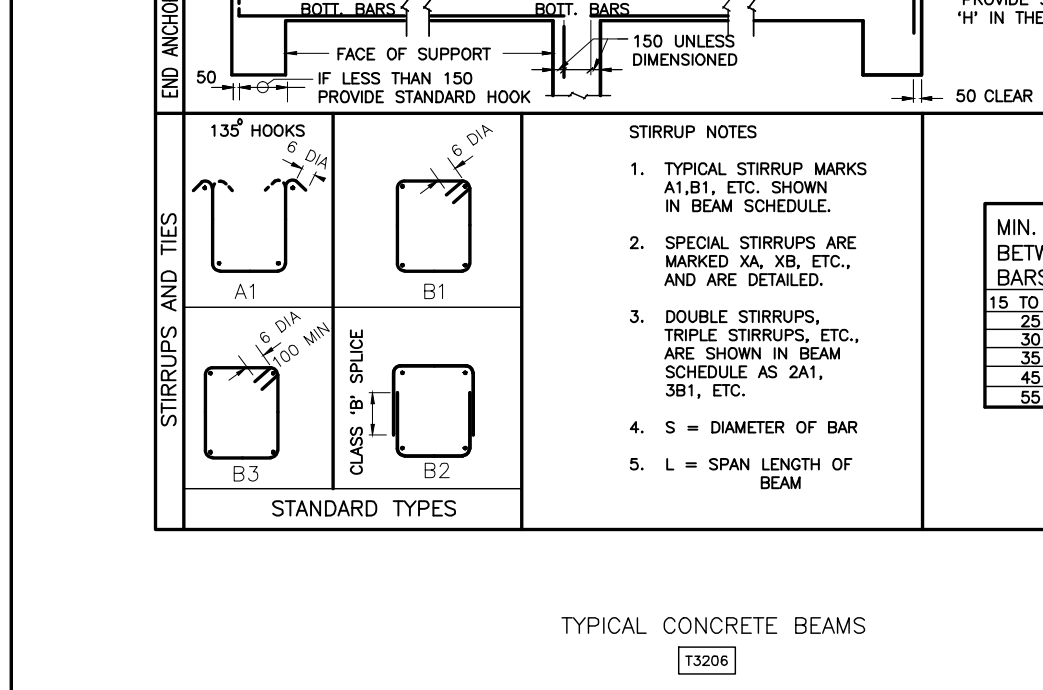
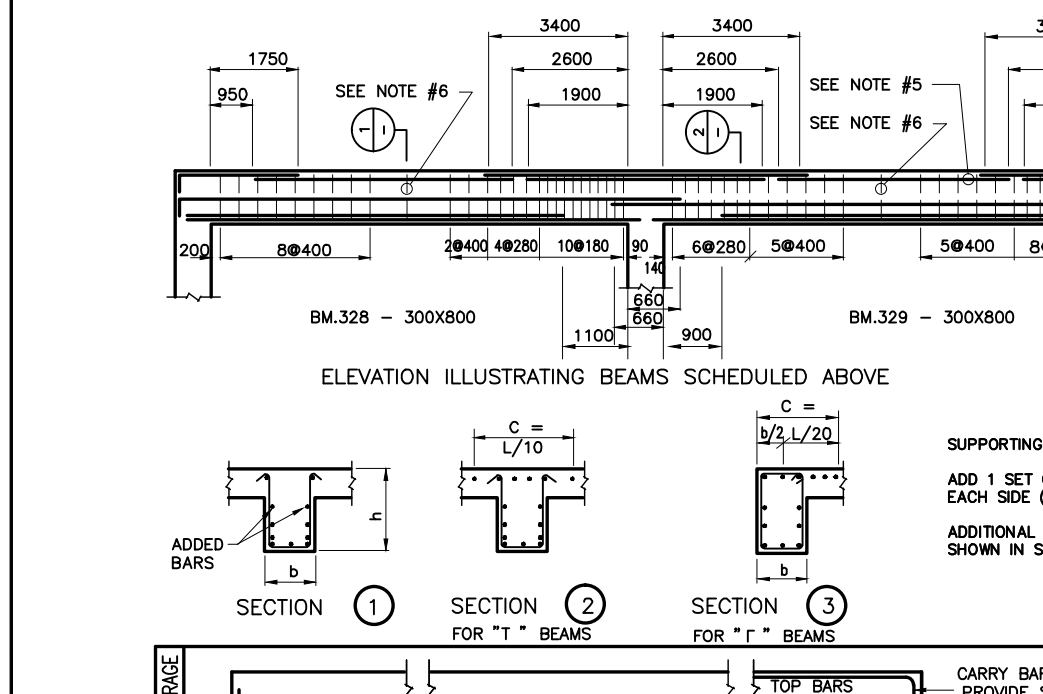
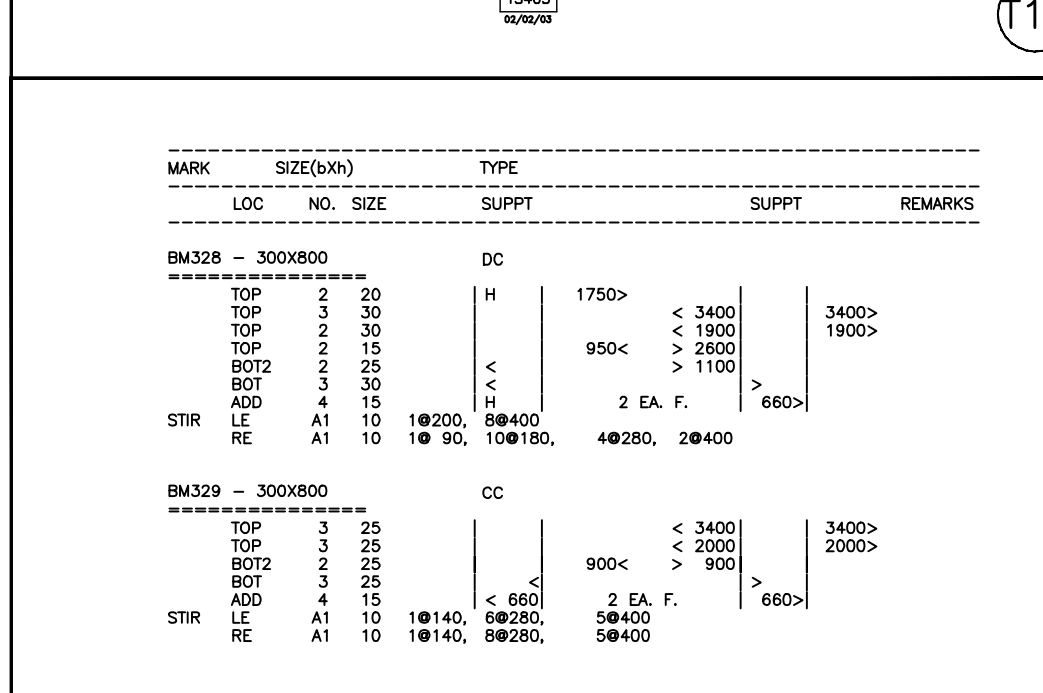
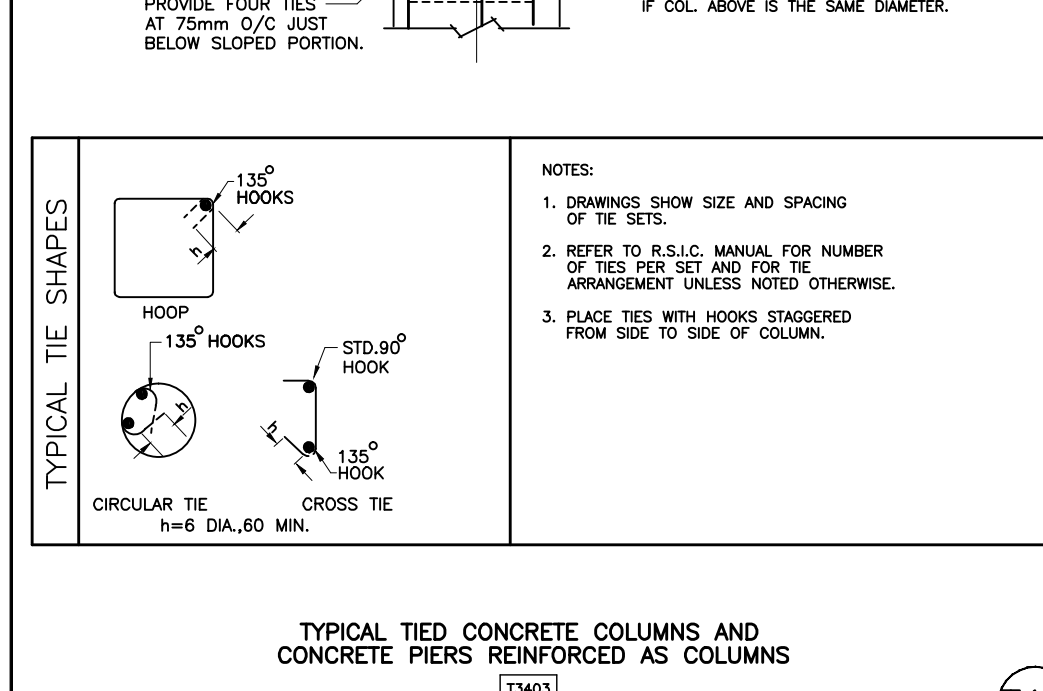
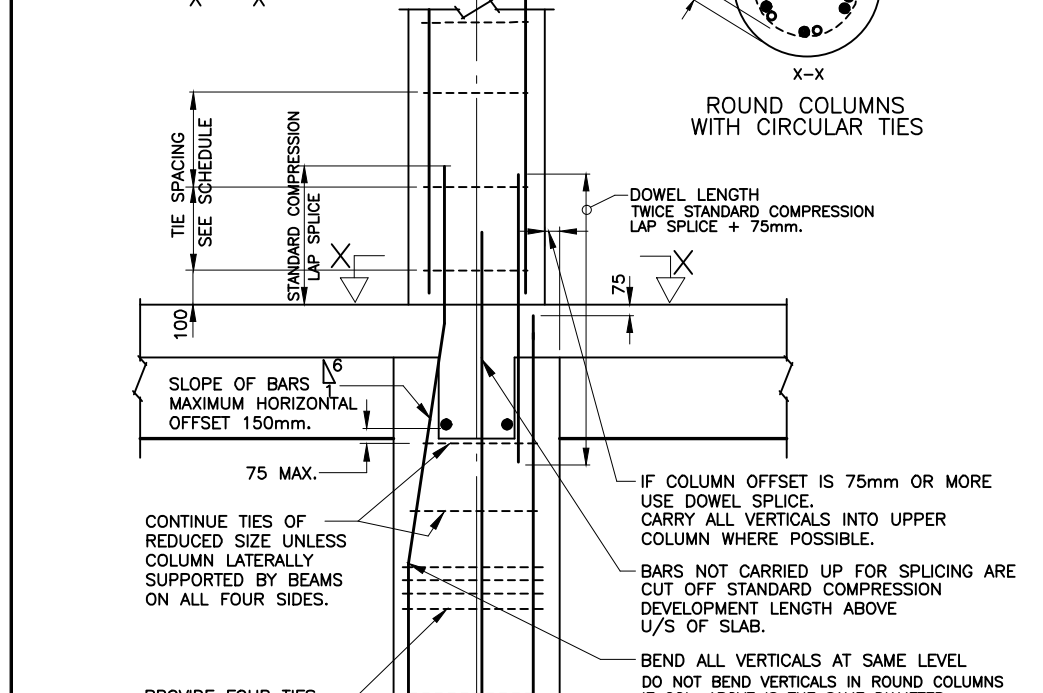
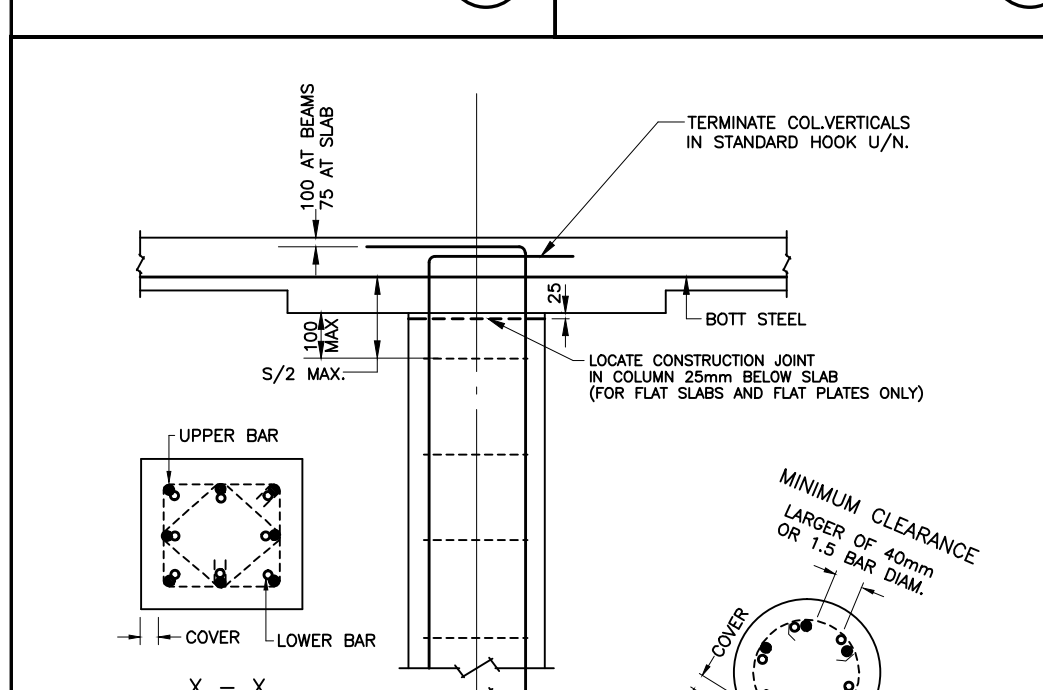
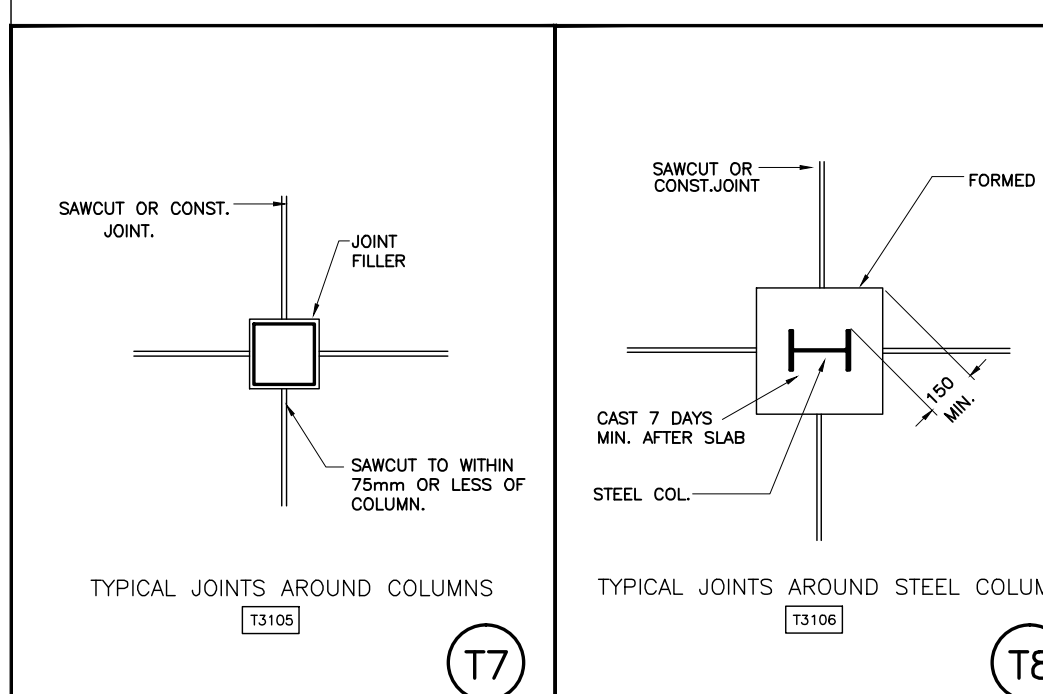
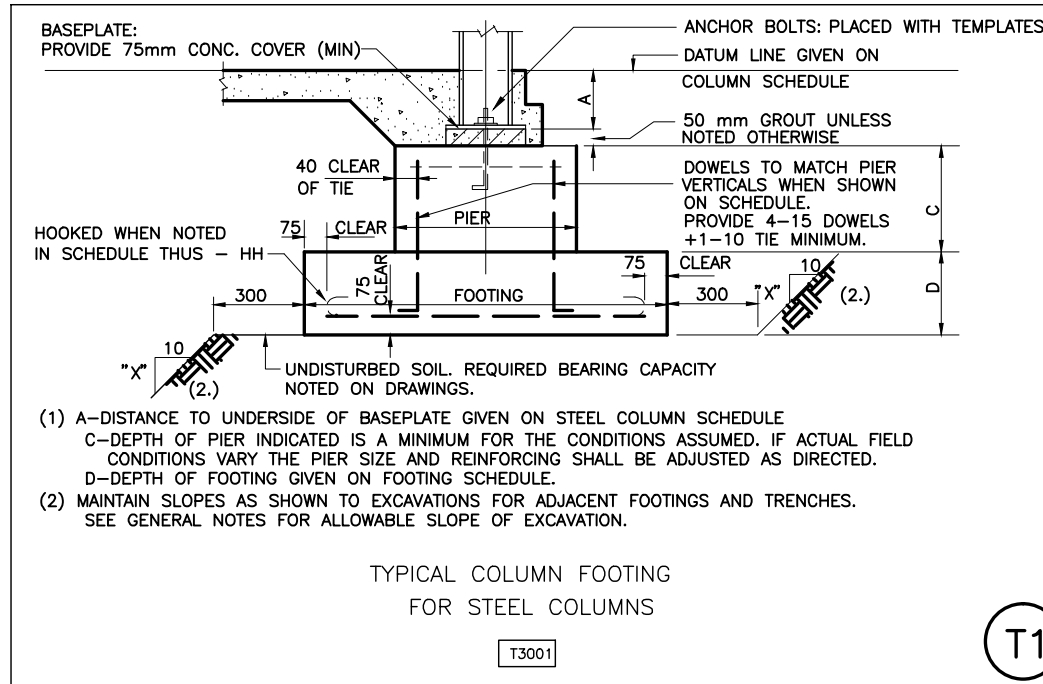
$S_g = 1.0$ kN/m ²	$S_r = 0.4$ kN/m ²
-------------------------------	-------------------------------
 - 24 HOUR RAINFALL: 150 mm
 - WIND PRESSURE AT GRADE LEVEL:

BLOCKS A, H & F-1/F-2	$q = C_e C_g C_p = 0.55 \times 0.9 \times 0.2 \times 1.3 = 1.29$ kN/m ²
PARKING P1 & P2	$q = C_e C_g C_p = 0.40 \times 0.9 \times 0.2 \times 1.3 = 1.08$ kN/m ²
 - EARTHQUAKE:

Block A:	$v_s f_u / U_R = 0.05 \times 1.68 \times 1.5 \times 0.6 / 1.5 = 0.050$
Block F-1/F-2:	$v_s f_u / U_R = 0.05 \times 2.8 \times 1.5 \times 0.6 / 1.5 = 0.072$
Block H:	$v_s f_u / U_R = 0.05 \times 1.9 \times 1.5 \times 0.6 / 1.5 = 0.057$
Parking P1:	$v_s f_u / U_R = 0.05 \times 1.7 \times 1.5 \times 0.6 / 1.5 = 0.034$
Parking P2:	$v_s f_u / U_R = 0.05 \times 1.3 \times 1.5 \times 0.6 / 1.5 = 0.078$

DRAWING LEGEND

- UNLESS OTHERWISE NOTED, DESIGN LOADS SHOWN ARE SPECIFIED (WORKING) LOADS. FOR POINT LOADS, IF ONLY ONE LOAD IS GIVEN, CONSIDER IT LINE LOAD.
- LIVE LOAD (kN/m²)
- DEAD LOAD (kN/m²)
- SUPERIMPOSED DL (EXCLUDING SELF-WEIGHT) (kN/m²)
- POINT LOAD (kN)
- FOR STEEL JOISTS IS DL + LL IN kN/m²
- FOR PRECAST CONCRETE IS DL + LL IN kN/m²
- FACTORED MOMENT (kN-m)
- FACTORED TORSION (kN-m)
- FACTORED SHEAR (kN)
- FACTORED VERTICAL REACTION (kN)
- FACTORED AXIAL LOAD (kN)
- + INDICATES TENSION
- INDICATES COMPRESSION
- DESIGNED MEMBER MARK
- DISTANCE TO TOP OF BEAM FROM DATUM ELEVATION OF FLOOR OR ROOF
- STEP DOWN FOOTING IN DIRECTION OF ARROWHEAD
- HOLES THROUGH STEEL BEAMS
- HOLES THROUGH CONCRETE BEAMS
- DOUBLE JOIST
- STEEL MEMBER CONCRETE FIREPROOFED
- CONCRETE INHAUCED DOWN TO BOTTOM FLANGE OF BEAM
- CONCRETE COMPOSITE STEEL BEAM
- FOR STEEL CONSTRUCTION C AND B DIMENSIONS ON PLAN ARE OFFSET DISTANCES FROM GRID LINE TO CENTRE LINE OF STEEL COLUMN OR BEAM IN DIRECTION OF ARROWHEAD
- FOR CONCRETE CONSTRUCTION C AND B DIMENSIONS ON PLAN ARE OFFSET DISTANCES FROM GRID LINE TO FACE OF CONCRETE COLUMN OR BEAM IN DIRECTION OF ARROWHEAD
- REINFORCING BAR HOOKED BOTH ENDS, ONE END
- RIGID FRAME MEMBER
- DRILLED CONCRETE ANCHORS
- DRILLED MASONRY ANCHORS
- EPOXY-GLUED REINFORCING
- ZINC-RICH PAINTED STEEL
- GALVANIZED STEEL
- COLUMN ABOVE
- CAMBER
- REVERSE CAMBER
- VERTICALLY SLOTTED CONNECTION TO ALLOW FOR DEFLECTION



CAISSON SCHEDULE (BEARING PRESSURE 2000 kPa)			
MARK	SHAFT DIA.	MAX. LOAD	
CAS-F1	914	1312 kN	
CAS-F2	1219	2334 kN	
CAS-F3	1524	3648 kN	
CAS-F4	1828	5249 kN	
CAS-F5	2134	7153 kN	

CAISSON CAP SCHEDULE				
MARK	SIZE	REINFORCING	DOWELS	REMARKS
CAP-F0	600 x 600		8-20V	
CAP-F1	800 x 800		8-25V	
CAP-F1A	800 x 800		8-25V	SEE SEC. F10/33301 FOR ADDITIONAL REINFORCEMENT
CAP-F2	1500 x 1500		16-30V	
CAP-F3	1500 x 1500	6-25 T&BEW		
CAP-F4	2200 x 2200	9-25 T&BEW		ADD DOWELS WHERE SHOWN ON PLAN
CAP-F5	2000 x 2000	6-25 T&BEW		SEE DETAIL FOR ADDED REINF.

CAISSON AND CAISSON CAP NOTES

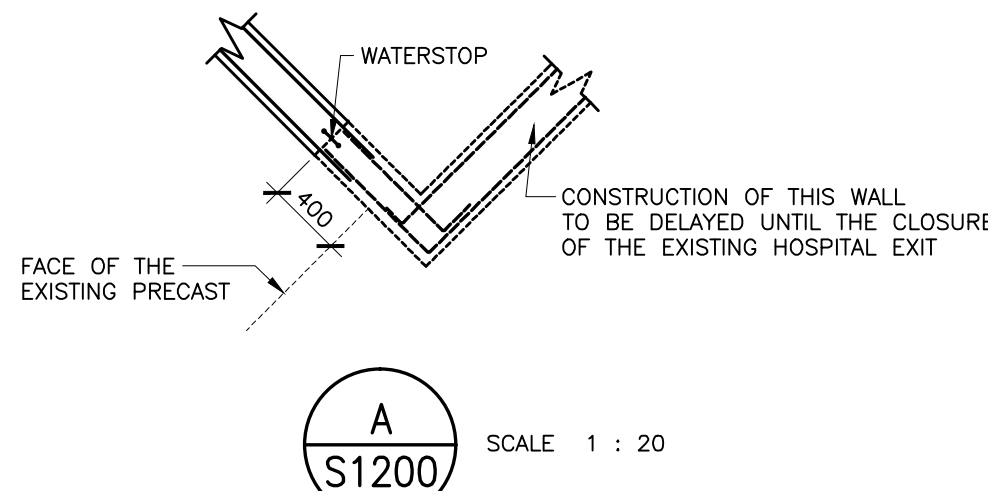
- EXTEND CAISSONS AS REQUIRED TO ACHIEVE THE ASSUMED ALLOWABLE BEARING CAPACITY OF 2000kPa.
- CENTRE CAISSONS UNDER COLUMNS ABOVE, UNLESS NOTED OTHERWISE.
- WHERE THERE ARE NO COLUMNS, CENTRE CAISSON ON WALL/GRADE BEAMS ABOVE, UNLESS NOTED OTHERWISE.
- FOR LOCATION OF CAISSONS AT WALL/GRADE BEAM CORNERS, SEE DETAIL T60 ON DRAWING S1002.
- FOR CAISSONS AT CORNERS OF SHEAR WALLS (W1 TO W10), SEE DETAILS AS ON DRAWING S4002.
- ASSUMED CAISSON LENGTH IS 3 TIMES THE CAISSON DIAMETER, UNLESS A DIFFERENT LENGTH, (MIN.) L=XX IS SHOWN ON PLAN.
- MINIMUM CAISSON LENGTH IS EQUAL TO 3 TIMES THE CAISSON DIAMETER.
- "TOP" ON PLAN INDICATES TOP OF CAISSON ELEVATION.
- SET TOPS OF INTERIOR CAISSONS AT ELEVATION 160.00 UNLESS NOTED.
- SET TOPS OF CAISSONS SUPPORTING PERIMETER GRADE BEAMS AT ELEVATION 159.85 UNLESS NOTED.
- CAISSON REINFORCING, WHERE REQUIRED, IS SHOWN ON PLAN. EXTEND REINFORCING FULL LENGTH OF CAISSONS. PROVIDE STANDARD HOOK AT TOP OF CAISSON (OR TOP OF CAP, GRADE BEAM, WHERE APPLICABLE). PROVIDE MIN. 150/500 TIES.
- FOR UNREINFORCED CAISSONS WITHOUT CAPS, PROVIDE 8-20x1200 LONG DOWELS BETWEEN EACH CAISSON AND THE STRUCTURE ABOVE.
- FOR UNREINFORCED CAISSONS WITH CAPS, SEE CAISSON CAP SCHEDULE FOR DOWELS BETWEEN CAISSON AND CAPS.
- CENTRE CAISSON CAPS ON COLUMNS ABOVE U/N.
- POUR CAISSON CAPS INTEGRALLY WITH GRADE BEAMS (WHERE APPLICABLE) U/N.
- INCREASE CAISSON CAP SIZE TO MATCH FLUSH WITH GRADE BEAMS (WHERE APPLICABLE).
- DOWELS FROM CAPS TO COLUMNS OR WALLS ABOVE TO MATCH COLUMN/WALL VERTICAL REINFORCING.

FOOTING SCHEDULE				
MARK	SIZE	REINFORCING	PIER SIZE	PIER REINFORCING
FF1	2000 x 2000 x 450 DEEP	12-15 BEW	800 x 800	8-25V
FF2	2600 x 2600 x 650 DEEP	14-20 BEW	800 x 800	8-25V
FF3	1000 x 1000 x 300 DEEP	4-10 BEW (HH)	800 x 800	8-25V

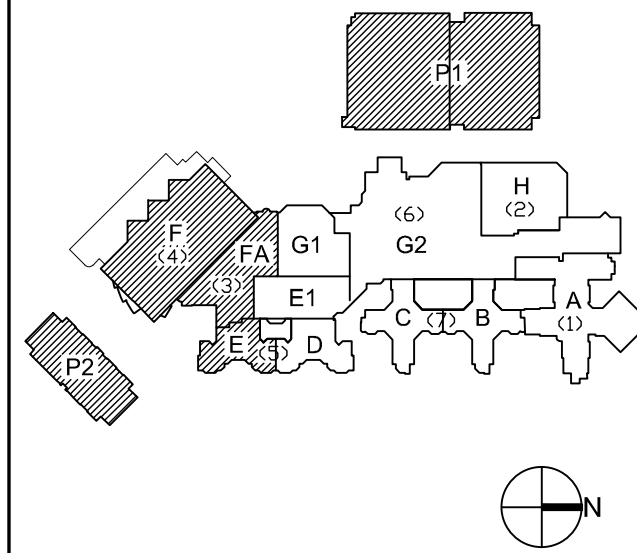
ASSUMED ALLOWABLE SOIL BEARING PRESSURE IS 400 kPa

FOUNDATION PLAN (LEVEL 0)

- SEE GENERAL REQUIREMENTS ON DRAWING S1001.
- SEE DRAWING S1001 AND S1002 FOR TYPICAL DETAILS.
- LEVEL 0 FLOOR DATUM ELEVATION IS 158.40.
- UNLESS OTHERWISE NOTED ON PLAN OR DETAILS, THE FOLLOWING DATA APPLY:
 - TOP OF SLAB IS 80.00 FROM DATUM ELEVATION.
 - FRAMED AREAS: DESIGN LIVE LOAD IS 4.8 kN/m².
 - PROVIDE CONCRETE WALL AND MASONRY WALL FOOTINGS AS PER TYPICAL DETAILS.
 - SET TOPS OF COLUMN FOOTINGS AT EL. 160.0.
 - SET TOPS OF WALL FOOTINGS AT EL. 160.8.
- GRADE BEAMS AND WALL BEAMS: SPLICE TOP REINFORCING AT MID-SPAN AND BOTTOM REINFORCING OVER SUPPORTS. USE CLASS A LAPS.
- PROVIDE 125 THICK CONCRETE SLAB-ON-GRADE WITH 152 x 152 MW18.7 / MW18.7 W.W.F. PLACED 50 mm BELOW TOP OF SLAB.
- PROVIDE BONDED CONCRETE TOPPING OVER DERESSED SLABS AS REQUIRED TO MAINTAIN FINISHED FLOOR LEVELS. REINFORCE WITH 102 x 102 MW18.7 / MW18.7 W.W.F. PLACED IN CENTRE OF TOPPING.
- "COL FC1" ON PLAN INDICATES 300x300 CONCRETE COLUMN, REINF. 20-25V, 100/300 TIES, U/S OF COLUMN EL. 160.6.
- "COL FC2" ON PLAN INDICATES 600 DIA. CONCRETE COLUMN, REINF. 12-25V, 100/300 TIES, U/S OF COLUMN EL. 160.6.
- FOR STEEL COLUMNS (AND CONCRETE COLUMNS SUPPORTING STEEL COLUMNS), SEE COLUMN SCHEDULE ON DWG. S4301.
- GROUT BASE PLATES AND BEARING PLATES PRIOR TO PLACING LOADS ON STEELWORK.



ALTERNATIVE: ALL CAISSONS SUPPORTING CANOPY COLUMNS ONLY CAN BE SUBSTITUTED WITH FOOTINGS F1'S U/S OF FOOTING ELEVATION TO MATCH BOTTOM OF CAISSON ELEVATION. NOTE THAT CONSTRUCTION OF THESE CAISSONS/F10'S HAS TO BE DELAYED AND COORDINATED WITH CLOSURE OF THE EXISTING HOSPITAL EXITS.



KEY PLAN

ISSUES

NO.	DESCRIPTION	DATE
E	ISSUED FOR COSTING	02-01-16
F	ISSUED FOR ISSUING PERMIT APPLICATION	02-03-06
G	BLOCK F/FA STRUCTURAL COMPONENT ONLY	02-03-22
H	ISSUED FOR NEW APPROVAL TO TENDER	02-03-22
I	BLOCK F/FA STRUCTURAL COMPONENT ONLY	02-06-14
J	ISSUED FOR COSTING	02-06-14
K	ISSUED FOR MOHLC REVIEW	02-09-08
L	ISSUED FOR MOHLC REVIEW	02-09-30
M	ISSUED FOR MOHLC REVIEW	02-09-30
N	ISSUED FOR TENDER	02-10-21
O	ISSUED FOR CONSTRUCTION	03-01-27

NO.	DESCRIPTION	DATE
CCO#1		03-01-27

REVISIONS

NO.	DESCRIPTION	DATE
1	THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS DO NOT SCALE THE DRAWINGS.	
2	THIS DRAWING SHALL NOT BE USED FOR CONSTRUCTION PURPOSES UNTIL ISSUED FOR CONSTRUCTION BY THE DESIGN PROFESSIONAL. NOISE SEAL IS AFFIXED TO THIS DRAWING AND NOISE SIGNATURE IS BELOW.	
3	DATE	
4	ISSUED FOR CONSTRUCTION	

Halsall
ENGINEERS • CONSULTANTS
2300-2300 YONGE ST., P.O. BOX 2385
TORONTO, ONTARIO, CANADA M4P 1E4
tel: (416) 487-5256 fax: (416) 487-9766
1-888-HALSALL halsall.com

134 Peter Street
Toronto, Ontario
M5V 2G9
Telephone: (416) 979-3666
Fax: (416) 979-3680



CREDIT VALLEY
THE CREDIT VALLEY HOSPITAL

PROJECT NAME:

THE CREDIT VALLEY HOSPITAL /
PEEL REGIONAL CANCER CENTRE

DRAWING TITLE:

FOUNDATION PLAN
(LEVEL 0)
BLOCK FA / F / E

SCALE:

1:200

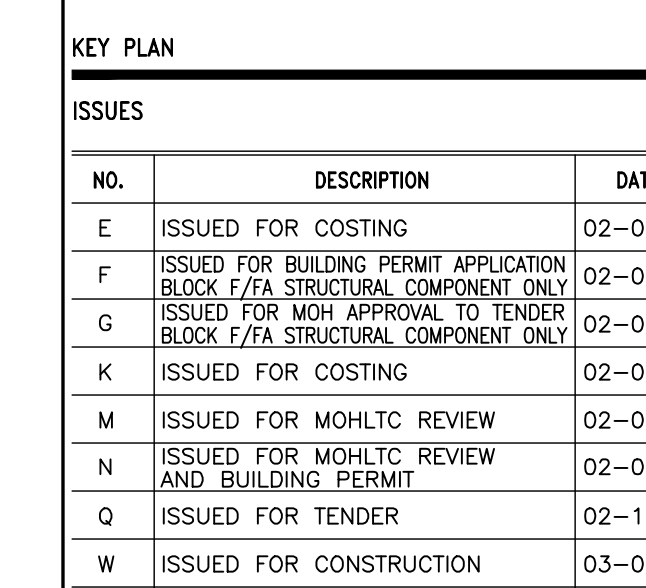
PROJECT NO.

01038

DWG NO.

S1200

W



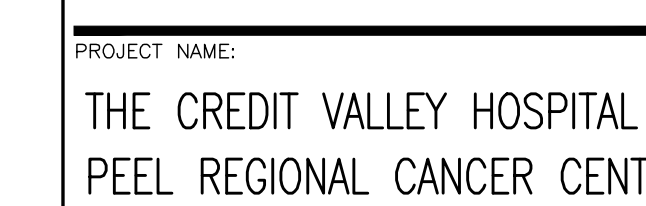
1	CCO#1	03-0
NO.	DESCRIPTION	DATE

THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS. DO NOT SCALE THE DRAWINGS.

THIS DRAWING SHALL NOT BE USED FOR CONSTRUCTION PURPOSES, ISSUED FOR CONSTRUCTION BY DESIGN PROFESSIONAL WHOSE NAME IS AFFIXED TO THIS DRAWING AND WHOSE SIGNATURE IS BELOW.

DATE _____

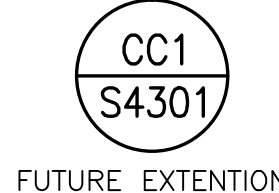
134 Peter St.
Toronto, Ont.
M5V
Telephone: (416) 979-3
Fax: (416) 979-3



PROJECT NO:	DWG NO:	
01038	S1201	VA



1. PROVIDE 4-19 mm DIAMETER ANCHOR BOLTS x 400 mm LONG PLUS 75 mm HOOK FOR EACH BASE PLATE, UNLESS OTHERWISE NOTED IN SCHEDULE.
2. SEE PLAN FOR COLUMN ORIENTATIONS.
3. COLUMN BOLTS TO BE GROUDED WITH 50 mm OF GROUT. SEE SPECIFICATIONS.
4. ALL COLUMN BOLTS TO BE PLATE DIMENSION GIVEN FIRST TO BE IN NORTH-SOUTH DIRECTION.
5. PROVIDE LEVELLING SCREWS FOR BASE PLATES.
6. WELD BASE PLATES TO COLUMNS TO DEVELOP ANCHOR BOLTS.
7. PROVIDE STANDARD WALL TIES TO ALL COLUMNS ABUTTING MASONRY AS PER TYPICAL DETAILS.
8. PROVIDE CAP PLATES ON ALL HES COLUMNS AND ON ALL OTHER COLUMNS WHERE REQUIRED FOR SUPPORT OF DECK ETC.
9. "CAP1" INDICATES 19 mm CAP PLATE FULLY WELDED ON TOP OF COLUMN.
10. FOUR CONCRETE COLUMN INFERIALLY WITH FOUNDATION WALLS INCREASE SIZE AS REQUIRED BY DESIGN. PROVIDE WITHIN THE SIZE SHOWN ON SCHEDULE. SEE PLAN AND SECTIONS FOR ADDITIONAL REINFORCEMENT.
11. CONCRETE COLUMNS SPECIFIED COMPRESSIVE STRENGTH IS $f_{ck} = 25$ MPa.

[illegible]

THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS. DO NOT SCALE THE DRAWINGS.

THIS DRAWING SHALL NOT BE USED FOR CONSTRUCTION PURPOSES UNTIL ISSUED FOR CONSTRUCTION BY THE DESIGN PROFESSIONAL WHOSE SEAL IS AFFIXED TO THIS DRAWING AND WHOSE SIGNATURE IS BELOW.

DATE _____

ISSUED FOR CONSTRUCTION _____

134 Peter Street
Toronto, Ontario
M5V 2G5
Telephone: (416) 979-3666
Fax: (416) 979-3680



SCALE: N. A.

PROJECT NO.	DWG NO:	
01038	S4301	W

